

# Optimization of cylindrical shells subjected to pitting corrosion

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The paper presents modelling of optimization process of thin-walled structures such as vertical cylindrical reservoirs subjected to pitting corrosion. The problem is formulated in terms of nonlinear mathematical programming. The function which is a product of its constituents is accepted as the optimization criterion. The choice of an optimal thickness of the reservoir shell along the height is determined from the conditions of its equal reliability.

## 1. INTRODUCTION

The surfaces of reservoir under pressure of a liquid and those operating in a corrosive medium are often subjected to pitting corrosion. In this case it is rather important to carry out an estimation of the strength and reliability of the structure, because the development of at least one corrosion pit which gives rise to a substantial hole causes the loss of its service capability and leads to serious emergency consequences. On the other hand, as mentioned in [1], reservoirs occupy the leading place in the volume of construction and in the amount of steel used for it. For this reason the problems of increasing their strength decreasing the weight of metal and the amount of labour for their manufacture are urgent. The development of a probability model of failure for thin-walled structures due to pitting corrosion was presented in [2]; it was supposed that the initial pitting size distribution along the surface of the structure and the time of appearance of the first hole are normal variates. Optimal design of type of structures using the criterion of average expected utility was discussed in [3].

## 2. FORMULATION OF THE OPERATION PROBLEM

The paper presents the modelling of a comprehensive approach to the design of optimal structures of this class from the utility point of view and the degree of the relation to its components. The product of the average expected utility function [4]:  $U(\mathbf{X}) = B(\mathbf{X}) - H_1(\mathbf{X}) - L(\mathbf{X})$  and characteristic coefficients of its constituents is accepted as an optimization criterion for the cylindrical shell

$$F = U(\mathbf{X})\mu(K, B)\mu(K, H_1)\mu(K, L), \quad (1)$$

where  $\mathbf{X}$  is a vector of variable parameters (from the set of structure states  $K$ );  $B(\mathbf{X})$  — average income expected from the service during its design period of life  $T$  with regard to a possible failure at the moment of time  $t_{\text{fail}} < T$ ;  $H_1(\mathbf{X})$  — the initial cost,  $L(\mathbf{X})$  — damage due to the failure of structure  $\mu(K, B)$ ,  $\mu(K, H_1)$  and  $\mu(K, L)$  are characteristic coefficients or membership functions of the design (their values vary within the range  $[0,1]$ ) which define the degree of the compliance of the structure with the present optimal parameters of the income, initial cost and loss, respectively. Modelling of this type of membership functions depends on the priority of every constituent factor

of the utility function. In this case they are equivalent, and the membership functions are accepted as follows:

$$\mu(K, B) = \begin{cases} 0, & \text{if } B \leq B_{\min}, \\ 1/2 + 1/2 \sin\{\pi[B - (B_{\text{opt}} + B_{\min})/2]/(B_{\text{opt}} - B_{\min})\}, & \text{if } B_{\min} \leq B \leq B_{\text{opt}}, \\ 1, & \text{if } B > B_{\text{opt}}. \end{cases}$$

$$\mu(K, L) = \begin{cases} 1, & \text{if } L \leq L_{\text{opt}}, \\ 1/2 - 1/2 \sin\{\pi[L - (L_{\text{opt}} + L_{\max})/2]/(L_{\max} - L_{\text{opt}})\}, & \text{if } L_{\max} \leq L \leq L_{\max}, \\ 0, & \text{if } L > L_{\max}. \end{cases}$$

$$\mu(K, H_1) = \begin{cases} 1, & \text{if } H_1 \leq H_{\text{opt}}, \\ 1/2 - 1/2 \sin\{\pi[H_1 - (H_{\text{opt}} + H_{\max})/2]/(H_{\max} - H_{\text{opt}})\}, & \text{if } H_{\text{opt}} \leq H_1 \leq H_{\max}, \\ 0, & \text{if } H_1 > H_{\max}, \end{cases}$$

where  $B_{\min}$ ,  $L_{\max}$  and  $H_{\max}$  are minimum and maximum permissible values,  $B_{\text{opt}}$ ,  $L_{\text{opt}}$ , and  $H_{\text{opt}}$  are suboptimal values of the income, loss and initial cost of the tank adopted by the designer, respectively.

The purpose of the optimization task is to find the vector of the structure parameters  $\mathbf{X}_{\text{opt}}$  that maximizes the function (1) with limitation of the reliability:

$$[F = U(\mathbf{X})\mu(K, B)\mu(K, L)] \Rightarrow \max, \quad P(T) \geq P_*, \quad (2)$$

where  $P(T)$  — the reliability,  $P_*$  — the value of assumed reliability.

The vector of variable parameters  $\mathbf{X}$  is assumed as follows:  $\mathbf{X} = \{x_1, x_2\}^T = \{n, T\}^T$ , where  $n$  — the number of constant thickness belts (sheets) into which the reservoir is divided;  $T$  — designed service life of the reservoir. The reservoir is under the influence of the hydrostatic internal pressure of a liquid, corrosive medium (such as petroleum products) of density  $\rho$ . The general reservoir dimensions: radius  $R$  and height  $H$  are known.

### 3. CALCULATION METHODS

Let us show the determination of the values included in (2). Income and loss due to the failure of the structure will be modelled according to [3]:

$$B(\mathbf{X}) = \int_0^T B^0(t) p_{\text{fail}}(t) dt, \quad (3)$$

where:  $B^0(t) = b(1 - e^{-rt})/r$  — income;  $b$  — annual income in the case there is no failure;  $r = \ln(1 + r')$ ;  $r'$  — interest on the capital;

$$p_{\text{fail}}(t) = P'_{\text{fail}}(t) = [1 - P(t)]' \quad (4)$$

is density of the failure probability of the structure during the period of its service.

The average value of losses (or a loss due to failure) up to the present time is modelled similarly:

$$L(\mathbf{X}) = \int_0^T L^0(t) p_{\text{fail}}(t) dt, \quad (5)$$

where:  $L^0(t) = L_t e^{-rt}$  — losses;  $L_t$  — total damage evaluated before the structure entered into service.

The initial cost of the structure is determined as [1]:

$$H_1(\mathbf{X}) = C_3 + C_m, \quad (6)$$

where  $C_3 = C_{om} + C_i + C_p$  — manufacturing cost;  $C_{om} = 1.07(\sum^n C_{npi}k_{npi}G_i + 1.5G)$  — cost of the materials;  $C_{npi}$  — wholesale price of sheets for the  $i$ -th part of the cost of main structure;  $G_i = 2\pi R h_i H \gamma / n$  — weight of the  $i$ -th part;  $C_i = C_{ob} + C_{cb} + C_n \cong 0.62\sqrt{Gn}$  — cost of the manufacture structure;  $C_{ob}, C_{cb}, C_n$  — cost of machining, erection welding and rolling;  $C_p$  — cost of structure, *FOB* destination  $C_p = 1.1406[(C_{om} + C_i)1.0054 + 2.66]$ ;  $C_m = 0.641\sqrt{Gn}$  — cost of assembling the reservoir.

Now let us determine the function of the shell reliability included in (2). The reliability of the shell structure is the probability of a random event consisting in the fact that no pitting formation will exceed the permissible level — in this case, the thickness of the sheet — during the present period of its operation  $0 \leq t \leq T$ . The equation of corrosion is modelled in the form:

$$dl_i/dt = \alpha + \beta\sigma_i, \quad (7)$$

where  $l_i$  — current depth of pitting;  $\alpha$  and  $\beta$  — constant coefficients;  $\sigma_i$  — effective stress in the  $i$ -th sheet. The solution of the equation (7) if:  $t = 0, l_i = l_{0i}$ , is as follows:

$$l_i = l_{0i} + (\alpha + \beta\sigma_i)t = l_{0i} + b_i t.$$

If we take the initial depth of pitting  $l_{0i}$  as a random value with a normal density  $q(l_{0i}) = \exp[-(l_{0i} - \bar{l}_{0i})^2 / 2\sigma_{l_{0i}}^2] / \sqrt{2\pi}\sigma_{l_{0i}}$  and take into account that  $b_i$  is a constant value, then  $l_i$  is a fixed random value. In this case the probability that no pitting will exceed the thickness of the  $i$ -th sheet  $h_i$  (taken as a constant value) is determined as follows [5]:

$$P_i^* = 1/\sqrt{2\pi\sigma_{l_{0i}}} \int_{-\infty}^{h_i - b_i t} \exp[-(l_{0i} - \bar{l}_{0i})^2 / 2\sigma_{l_{0i}}^2] dl_{0i} = 0.5 + \Phi(a_i),$$

where  $a_i = (h_i - b_i t - \bar{l}_{0i}) / \sigma_{l_{0i}}$ ,  $\Phi(a_i) = 1/\sqrt{2\pi} \int_0^{a_i} e^{-z^2/2} dz$  — Laplace integral.

If we denote the total number of pitting formations on the surface of the whole reservoir by  $N$  and assume the thickness along the height of the shell in such a way that the reliability of each sector is constant (equally reliable), then the reliability of the whole reservoir is determined by the expression:

$$P = (P_i^*)^N.$$

Assuming the permissible level of reliability  $P_*$  and the total number of pitting formations, it is easy to calculate the required values of thickness  $h_i$  complying with the principle of equal reliability of the reservoir. In this case:  $P_i^* = (P_*)^{1/N}$  and  $a = a_i = (h_i - b_i t - \bar{l}_{0i}) / \sigma_{l_{0i}} = \text{const}$ , where  $a$  is found from the condition:

$$(P_*)^{1/N} = \Phi(a).$$

Taking  $\bar{l}_{0i} = c_1 h_i$ ,  $\sigma_{l_{0i}} = c_2 h_i$ , we get:

$$a = (h_i - c_1 h_i - b_i t) / c_2 h_i, \quad b_i = \alpha + \beta\sigma_i. \quad (8)$$

As the effective stress in the  $i$ -th sheet is  $\sigma_i = \rho H i R / n h_i$ , the unknown quantities  $h_i$  are determined as follows:  $h_i = K + \sqrt{k^2 + p}$ ,  $i = 1, \dots, n$ , where  $k = \alpha t / 2(1 - c_1 - c_2 a)$ ;  $p = \beta H i \rho R t / n(1 - c_1 - c_2 a)$ .

Having determined the function of reliability and having substituted the value  $p_{\text{fail}}$  in (3) and (5), obtained according to (4), we can find the expressions of the income and the damage due to the failure of the structure:

$$B(\mathbf{X}) = bN \left\{ [0.5 - \Phi(a)] - e^{-b_1 + a_1^2/2} [0.5 - \Phi(a - a_1)] \right\} / r. \quad (9)$$

$$L(\mathbf{X}) = L_T N e^{-b_1 + a_1^2/2} [0.5 - \Phi(a - a_1)], \quad (10)$$

where  $b_1 = r h_i (1 - c_1) / b_i$ ,  $a_1 = r c_2 h_i / b_i$ . It is necessary to note that  $a_1 = \text{const}$  and  $b_1 = \text{const}$  at any value of its results from the Eq. (8).

Having calculated the current value of  $B(\mathbf{X})$ ,  $H_1(\mathbf{X})$  and  $L(\mathbf{X})$ , we turn our attention directly to the reservoir optimization.

In case of complex, multiextremal tasks of nonlinear programming such as (2), it is advisable to use one of the effective algorithms of the random search method [6]. This algorithm, based on the global random search, uses the idea of the controllable selection of test points and multiple lowering to the local extreme. The algorithm and the program differ from a similar kind of algorithms by the method of modelling the prospective direction of the search. The process of the random search realized in the program is described in the form:

$$\bar{\mathbf{X}}_\xi^{(k+p)} = \bar{\mathbf{X}}_0^{(k)} \pm \bar{S} \sum -; \quad \bar{S} = \begin{cases} \gamma_1 S, & \text{if } F(\bar{\mathbf{X}}_\xi^{(k+p)}) < F(\bar{\mathbf{X}}_0^{(k)}); \\ \gamma_2 S, & \text{if } F(\bar{\mathbf{X}}_\xi^{(k+L_p)}) \geq F(\bar{\mathbf{X}}_0^{(k)}). \end{cases}$$

Here  $\sum -$  is a single random uniformly distributed vector;  $\gamma_1 \geq 1$ ;  $\gamma_2 < 1$ ;  $\gamma_1 \gamma_2 > 1$  — constants of tension (contraction) of the search hypercube  $S$ ;  $p = \{1, 2, \dots, L_p\}$  — number of random realizations of vector  $\bar{\mathbf{X}}_\xi$  at constant  $\bar{S}$ ; signs  $\pm \bar{S} \sum -$  denote the realization of the double return of the test random point  $\bar{\mathbf{X}}_\xi$ ;  $\bar{\mathbf{X}}_0^{(k)}$  — parameters corresponding to the lowest value obtained at  $k$ -th stage of the search  $F(\bar{\mathbf{X}}_0^{(k)})$ .

#### 4. DESIGN EXAMPLE

As an illustration let us consider the optimization of the reservoir model having the following initial data:  $R = 2$  m;  $H = 4$  m;  $N = 48$ ;  $\alpha = 0.06$  cm/y;  $\rho = 0.8 \cdot 10^3$  kg/m<sup>3</sup>;  $c_1 = 0.2$ ;  $c_2 = 0.01$ ;  $P_* = 0.99$ ;  $b = 10^4$ ;  $L_T = 10^4$ ;  $r' = 10\%$ ;  $B_{\text{opt}} = 1200$ ;  $B_{\text{min}} = 800$ ;  $L_{\text{max}} = 10$ ;  $L_{\text{opt}} = 5$ ;  $H_{\text{max}} = 250$ ;  $H_{\text{opt}} = 180$ .

Four values of different coefficients of correlation between the corrosion and stress  $\beta$  have been modelled:  $\beta_1 = 0.085$  cm<sup>3</sup>/T·year;  $\beta_2 = 0.17$  cm<sup>3</sup>/T·year;  $\beta_3 = 0.24$  cm<sup>3</sup>/T·year;  $\beta_4 = 0.34$  cm<sup>3</sup>/T·year. The assumed ranges of the variable parameters are:  $2 \leq n \leq 10$ ; 5 years  $\leq T \leq 40$  years.

The results of the numerical experiment on the optimization of reservoir shell are given in the Table 1.

Table 1

$\beta$ [ $\frac{\text{cm}^3}{\text{T} \cdot \text{y}}$ ]	$n$	$T$ [y]	$h_1$ [cm]	$h_n$ [cm]	$B(T, \mathbf{X})$	$H_1(\mathbf{X})$	$L(T, \mathbf{X})$	$U(T, \mathbf{X})$	$\beta_3$ [ $\frac{\text{cm}^3}{\text{T} \cdot \text{y}}$ ]	$\mu(K, B)$	$\mu(K, L)$	$\mu(K, H_1)$	$F$
0.085	10	27.93	2.191	2.194	969.77	217.62	7.08	745.06	0.085	0.3824	0.6304	0.4413	79.263
0.17	10	27.993	2.197	2.2	969.45	218.25	7.11	744.08	0.17	0.3812	0.6212	0.4272	75.27
0.25	10	28.094	2.205	2.21	969.42	219.16	7.11	743.14	0.25	0.381	0.6204	0.407	71.52
0.34	10	28.08	2.204	2.213	968.5	219.21	7.2	742.09	0.34	0.3776	0.5936	0.406	67.53

#### 5. CONCLUSION

It is evident that the increase of the coefficient  $\beta$  has little influence on the optimal thickness of the reservoir (this is connected with the fact that the effective stresses in it are small). Thus, the

optimal service life of the reservoir is  $T = 28$  years, when the number of constant thickness belts  $n$  in all variants achieves its maximum. Due to this fact, the change of thickness  $h_i$  along the height found from the principle of equal reliability of the reservoir shell, is practically insignificant.

The use of the principle of equal reliability allowed to eliminate in optimization the limitation of reliability given in the statement (2). In conclusion, we can note that the type of characteristic functions and the limits set in them can be corrected in the course of search for optimal decisions.

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1. INTRODUCTION

Loading devices of cylindrical shells, such as pneumatic and hydraulic test and wind tunnel devices are distinguished. Their load is characterized by different types of loading, while the structural load intensity is not low.

Fundamental structural problems are to be solved when the results of pages 207 and 214 by distinguishing the term "load" and "deflection". The variation character of the loading process is near the applied load to the structure, on the contrary, deflection is dependent on the geometry of the structure. The kind of loading device can vary, but it is best defined by the load intensity, we focus on that type of a variable load, when the load is given by the load-deflection function.

Dead-type loading occurs used to other than load to be adapted to the geometry of the structure, namely, of a constant or linear variation load-deflection function. In this case, during a loading process the load  $P$  can be increased or decreased. In a constant load process, namely  $P = P(X)$ , where  $P(X)$  is the load-deflection function, while the load-deflection function is characterized by  $f = f(X)$ .

Variable or configuration-dependent type loading describes the applied load to be dependent on the occurring deflection, characterized by a variable, linear or nonlinear load-deflection function. In this case,  $P = P(X, y) = M_0 + f(y)$ , namely, the load is divided into two parts: the constant part  $M_0$  governed by the load parameter  $P_0$  and the linear range variation part  $f(y)$  is considered as a linear or nonlinear function. During a loading process, the structural part of the load can be increased or decreased by the load parameter  $P_0$ , while the original characteristics of the load-deflection function of the deflection-structure part is characterized by  $f(y) = f(y)$  and  $f(y)$  is the linear variable load-deflection function, can be specified as  $P = P_0 + f(y)$ , where  $P_0 = P_0$  and  $f = f$  is the constant load-deflection characteristic of  $P = P(X, y) = P_0 + f(y)$ , where  $P_0 = P_0$  and  $f = f$